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SPECIAL INSPECTIONS			
1. Special Inspections shall be performed in accordance with Section 1705 of 2021 IBC. An independent testing agency shall be employed to provide Special Inspections during construction on the types of work listed under Section 1705. The following areas of work require Special Inspections in accordance with 2021 IBC. 2. Refer to project specification for additional quality control/quality assurance requirements. 3. Construction Manager/Contractor shall coordinate any additional Special Inspection requirements with the Owner and applicable building authorities. 4. Special Inspections are not the responsibility of the Structural Engineer of Record. 5. Special Inspections shall be paid for directly by the Construction Manager. 6. Copies of all Special Inspections Reports shall be emailed to the SEOR Andrew Deschenes, P.E., (asd@tswstructural.com) or their designate within seven (7) calendar days of completing the individual inspection(s).			
STRUCTURAL STEEL (IBC 1705.2.1, 1705.13.1 & 1705.14.1)			
PRIOR TO WELDING (TABLE N5.4-1, AISC 360-16; TABLE J6-1, AISC 341-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Verify welding procedures (WPS) and manufacturer certifications for welding consumable available	X	-----	-----
Verify type and grade of material.	-----	X	For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Welder identification	-----	X	A system shall be maintained by which a welder who has welded a joint or member can be identified. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Fit-up groove welds	-----	X	Verify joint preparation, dimensions, cleanliness, tacking, and backing. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Access holes	-----	X	Verify configuration and finish. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Fit-up of fillet welds	-----	X	Verify dimensions, cleanliness, and tacking. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Check welding equipment	-----	X	-----
Welder qualification records and continuity records	-----	X	-----
DURING WELDING (TABLE N5.4-2, AISC 360-16; TABLE J6-2, AISC 341-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Use of qualified welders	-----	X	Verify that welders are appropriately qualified. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Control and handling of welding consumables	-----	X	Verify packaging and exposure control. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Cracked tack welds	-----	X	Verify welding does not occur over cracked tack welds. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Environmental conditions	-----	X	Verify wind speed within limits, precipitation and temperature. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
WPS followed	-----	X	Verify settings on welding equipment, travel speed, welding materials, shielding gas type/flow rate, preheat applied, interpass temperature maintained, and proper position. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Welding techniques	-----	X	Verify interpass and final cleaning, each pass within profile limitations, and quality of each pass. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Steel headed stud anchors	-----	X	Verify placement and installation.
AFTER WELDING (TABLE N5.4-3, AISC 360-16; TABLE J6-3, AISC 341-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Welds cleaned	-----	X	Verify welds properly cleaned. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Size, length, and location of welds	X	-----	-----
Welds meet visual acceptance criteria	X	-----	Verify crack prohibition, weld/base metal fusion, crater cross section, weld profiles, weld size, undercut, and porosity meet visual acceptance criteria.
Arc strikes	X	-----	-----
k-area	X	-----	-----
Backing & weld tabs removed and finished, and fillet welds added (if required)	X	-----	-----
Repair activities	X	-----	-----
Document acceptance or rejection of welded joint/member	X	-----	-----
Placement of reinforcing or contouring fillet welds	X	-----	Only required in components of seismic force resisting system.
Weld access holes	-----	X	After rolled heavy shapes are welded, visually inspect the weld access hole for cracks.
Prohibited welds	X	-----	Verify no prohibited welds have been added without approval of the EOR.
OTHER STEEL INSPECTIONS (SECTION N5.7 & N5.8, AISC 360-16; TABLES J8-1 & J10-1, AISC 341-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Structural steel details (fabricated steel or steel frames)	N/A	N/A	Verify compliance with the details in construction documents in items including: braces, stiffeners, member locations, and proper application of joint details at each connection. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Anchor rods and other embedments supporting structural steel	N/A	N/A	Verify compliance with construction documents. Verify diameter, grade, type, length of anchor rod or embedment item, and extent or depth of embedment prior to placement of concrete. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Reduced beam sections (RBS)	N/A	N/A	For seismic force resisting system components: Verify contour and finish as well as dimensional tolerances.
Protected zones	N/A	N/A	For seismic force resisting system components: Verify that no holes or unapproved attachments are made within the protected zone.
H-piles	N/A	N/A	For seismic force resisting system components: Verify that no holes or unapproved attachments occur within the protected zones of piling.
Galvanized structural steel	-----	X	Verify exposed cut surfaces of galvanized structural steel main members and exposed corners of rectangular HSS have no cracks subsequent to galvanizing.

STRUCTURAL STEEL (CONT.) (IBC 1705.2.1, 1705.13.1, & 1705.14.1)			
PRIOR TO BOLTING (TABLE N5.6-1, AISC 360-16; TABLE J7-1, AISC 341-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Manufacturer's certifications	X	-----	Verify certifications available for fastener materials.
Fasteners marked	-----	X	Verify marked in accordance with ASTM requirements. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Fastener selection	-----	X	Verify proper selection for joint detail including grade, type, and bolt length if threads excluded from shear plane. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Bolting procedure	-----	X	Verify proper bolting procedure selected for joint detail. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Connecting surfaces	-----	X	Verify connecting elements, including the appropriate faying surface condition and hole preparation, if specified, meet applicable requirements. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Pre-installation verification testing by installation personnel	X	-----	Observe and document for fastener assemblies and methods used.
Fastener storage	-----	X	Verify proper storage provided for bolts, nuts, washers, and other fastener components. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
DURING BOLTING (TABLE N5.6-2, AISC 360-16; TABLE J7-2, AISC 341-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Position of fasteners	-----	X	Verify fastener assemblies, of suitable condition, are placed in all holes and washers, if required, are positioned as required. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Joint brought into snug-tight condition prior to the pretensioning operation	-----	X	For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Fastener components not turned by the wrench are prevented from rotating	-----	X	For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
Pretensioning of fasteners	-----	X	Fasteners are pretensioned in accordance with the RCSC specification, progressing systematically from the most rigid point toward the free edges. For components of seismic force resisting system, perform on a random, daily basis per AISC 341-16 Section J5.1.
AFTER BOLTING (TABLE N5.6-3, AISC 360-16; TABLE J7-3, AISC 341-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Document acceptance or rejection of bolted connections	X	-----	-----
STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL (IBC 1705.2)			
STEEL ROOF AND FLOOR DECKS (IBC TABLE 1705.2.2/SDI QA/QC 6.1)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Material verification of cold-formed steel deck	-----	X	Verify deck materials are represented by appropriate mill certifications.
Floor and roof deck welding	-----	X	Verify weld meets acceptance criteria of AWS D.3. Verify welder qualifications.
Floor and deck mechanical fasteners	-----	X	Verify fastener installation in accordance with SDI.
Deck installation	-----	X	Verify deck installation in accordance with applicable drawings and documents.
COLD-FORMED STEEL CONSTRUCTION (IBC 1705.2.2, 1705.2.4, 1705.12.2, & 1705.13.3)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Trusses spanning > 60-feet	N/A	N/A	Verify temporary and permanent truss bracing is installed in accordance with approved truss package.
Welding in wind-force-resisting systems or seismic-force-resisting systems	-----	X	Verify proper screw attachment, bolting, anchoring and other fastening of shear walls, diaphragms, drag struts, braces, shear panels and holdowns. See IBC 1705.12.2 for exceptions.
Floor and roof deck welds	-----	X	Verify weld meets acceptance criteria of SDI QA/QC. Verify welder qualifications.

CONCRETE CONSTRUCTION (IBC 1705.3; TABLES J9-2 & J9-3, AISC 341-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Reinforcing steel, including prestressing tendons	-----	X	Verify, prior to placing concrete, reinforcing is of specified type, grade and size; free of oil, dirt and rust; located and spaced properly; hooks, bends, ties, stirrups and supplemental reinforcement placed correctly; lap lengths, stagger and offsets provided; and all mechanical connections installed per the manufacturer's instructions and/or evaluation report.
Cast-in anchors	-----	X	Verify anchor installation complies with ACI 318: 17.8.2.
Post-installed anchors	-----	X	All post-installed anchors shall be specially inspected as required by the approved ICC-ES report. Anchors installed horizontally or in upwardly inclined orientations to resist tension loads require continuous inspection per ACI 318: 17.8.2.4. Verify all other mechanical and adhesive anchors comply with ACI 318: 17.8.2.
Use of required mix design	-----	X	Verify mixes comply with the approved construction documents: ACI 318: Ch. 19, 26.4.3, 26.4.4 and IBC 1904.1, 1904.2.
Concrete sampling for strength tests, slump, air content, and temperature	X	-----	Verify sampling in accordance with ASTM C172 and ASTM C31. See ACI 318: 26.12 for evaluation and acceptance of concrete. See ACI 318: 26.5 for mixing requirements of concrete.
Concrete & shotcrete placement	X	-----	Verify proper application techniques. See ACI 318: 26.5.
Curing temperature and techniques	-----	X	Verify concrete surface temperature (other than high-early-strength) is kept >50°F in moist condition for at least 7 days after placement unless accelerated curing is used. High-early-strength concrete shall be kept >50°F in moist condition for at least 3 days unless accelerated curing is used. Verify compliance with cold weather requirements in ACI 318: 26.5.4 or hot weather requirements in ACI 318: 26.5.5, whichever is applicable.
Pre-stressed concrete	N/A	N/A	Verify application of prestressing force and grouting of bonded prestressing tendons in accordance with ACI 318: 26.10.
Erection of precast concrete	N/A	N/A	Verify all precast elements are lifted, assembled and braced in accordance with the approved construction documents. See ACI 318: 26.9.
Strength verification	-----	X	Verify adequate strength has been achieved prior to the removal of shores and forms or the stressing of post-tensioned tendons. See ACI 318: 26.11.2.
Formwork	-----	X	Verify forms are placed plumb and conform to the shapes, lines, and dimensions of the members as required by the approved construction documents. See ACI 318: 26.11.1.2.
Limits on water added at the truck or pump	-----	X	Verify during concrete placement. Applicable to composite construction in seismic force resisting system components. Perform on a random daily basis per AISC 341-16 Section J5.1. See Table J9-2, AISC 341-16.
Proper placement techniques to limit segregation	-----	X	Verify during concrete placement. Applicable to composite construction in seismic force resisting system components. Perform on a random daily basis per AISC 341-16 Section J5.1. See Table J9-2, AISC 341-16.
Verify installation of the embedded parts, completion of the continuity of reinforcement across joints, and completion of connections in the field	X	-----	For precast concrete diaphragm connections or reinforcement at joints classified as moderate or high deformability elements (MDE or HDE) in structures assigned to seismic design category C, D, E, or F. See ACI 318:26.13.1.3 and ACI 550.5.
Verify installation tolerances of precast concrete diaphragm connections	N/A	N/A	See ACI 550.5 for compliance.

INSPECTION OF FABRICATORS (IBC 1704.2)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Verify fabricator maintains detailed fabrication and quality control procedures	-----	X	See IBC 1704.2.5.1.
Submittal of certificate of compliance	-----	X	Where work is done on premises of "Approved" fabricator, Fabricator shall submit a Certificate of Compliance to the building official stating work was performed in accordance with the approved construction documents. See IBC 1704.2.5.1.

SOILS CONSTRUCTION (IBC 1705.6)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Verify subgrade is adequate to achieve design bearing capacity	-----	X	Prior to placement of concrete; per Geotechnical Report.
Verify excavations extend to proper depth and material	-----	X	Prior to placement of compacted fill or concrete; per Geotechnical Report
Verify subgrade has been appropriately prepared prior to placing compacted fill	-----	X	Prior to placement of compacted fill; per Geotechnical Report
Perform classification and testing of compacted fill materials	-----	X	All materials shall be checked at each lift for proper classifications and gradations not less than once for each 10,000 sq. ft. of surface area unless otherwise noted; per Geotechnical Report
Verify proper materials, densities and lift thicknesses	X	-----	During placement and compaction on compacted fill; per Geotechnical Report

MASONRY CONSTRUCTION (IBC 1705.4)			
PRIOR TO CONSTRUCTION (ARTICLE 1.5, TMS 602-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Review material certificates, mix designs, test results and construction procedures	-----	X	Verify materials conform to requirements of approved construction documents. Mix design, test results, material certificates, and construction procedures submitted for review. Mortar mix designs conform to ASTM C 270; grout conforms to ASTM C 476. Material certificates provided for: reinforcement, anchors, ties, fasteners, and metal accessories; masonry units; mortar and grout materials. Construction procedures for cold-weather or hot-weather construction reviewed. Qualification of field testing personnel, and special inspector reviewed.
AS CONSTRUCTION BEGINS (TABLE 4, TMS 602-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Proportions of site-prepared mortar	-----	X	Verify mortar is type and color specified on construction documents, conforms to ASTM C 270, and is mixed in accordance with Article 2.6 A and Article 2.6 C of TMS 602-16.
Grade, type, and size of reinforcement, connectors, anchor bolts, and prestressing tendons and anchorages	-----	X	Verify reinforcement is placed in accordance with Article 3.4 of TMS-602-16. Prestressing tendons placed per Article 3.6 A.
Sample panel construction	-----	X	Verify sample panel complies with Article 1.6 D of TMS-602-16.
DURING MASONRY CONSTRUCTION (TABLE 4, TMS 602-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Size and location of structural elements	-----	X	Verify locations of structural elements comply with approved plans. Confirm tolerances meet the requirements of Article 3.3 F of TMS 602-16.
Type, size, and location of anchors, including other details of anchorage of masonry to structural members, frames, or other construction.	-----	X	Verify anchorages and connections are provided per approved plans, Section 1.2 (1.e), 6.1.4.3, and 6.2.1 of TMS 402-16. Continuous inspection required for Risk Category IV buildings.
Welding of reinforcement	N/A	N/A	Verify welded splice has bars butted and welded to develop at least 125% of yield strength of bar in tension or compression. See Section 6.1.6.1.2 of TMS 402-16.
Preparation, construction, and protection of masonry during cold weather (<40°F) or hot weather (>90°F).	-----	X	Verify cold-weather construction performed in accordance with Article 1.8 C of TMS 602-16 and hot weather construction per Article 1.8 D of TMS 602-16.
Observation of preparation of grout specimens, mortar specimens, and/or prisms	-----	X	Confirm specimen/prism preparation performed as required by Article 1.4 of TMS 602-16. Continuous inspection is required for Risk Category IV buildings.
Placement of masonry unit and construction of mortar joints	-----	X	Verify placement in accordance with Article 3.3 B of TMS 602-16.
Materials and procedures with the approved submittals	-----	X	Verify materials and procedures conform to approved submittals. See Article 1.5 of TMS 602-16.
PRIOR TO GROUTING (TABLE 4, TMS 602-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Grout space	-----	X	Verify grout space is free of mortar droppings, debris, loose aggregate, and other deleterious materials and cleanouts are provided per Article 3.2 D and 3.2 F of TMS-602-16. Continuous inspection is required for Risk Category IV buildings.
Placement of reinforcement, connectors, and anchor bolts	-----	X	Verify reinforcement, joint reinforcement, wall ties, anchor bolts and veneer anchors are installed in accordance with the approved construction documents, Section 6.1, 6.3.1, 6.3.6, and 6.3.7 of TMS 402-16, and Articles 3.2 E, and 3.4 of TMS 602-16. Continuous inspection is required for Risk Category IV buildings.
Proportions of site-prepared grout.	-----	X	Verify grout is proportioned per ASTM C 476 and has a slump between 8-11 inches. Self-consolidated grout shall not be proportioned onsite. See Article 2.6 B of TMS 602-16.
MINIMUM TESTING (TABLE 3, TMS 602-16)			
Verification & Inspection	Continuous	Periodic	Detailed Instructions
Verification of slump flow and Visual Stability Index (VSI) for self-consolidating grout	-----	X	Compressive strength tests should be performed in accordance with ASTM C 1019; slump flow and VSI performed in accordance with ASTM C 1611.
Verification of fm and fAAC	-----	X	Determine compressive strength for each wythe by "unit strength method" or by the "prism test method" as specified in Article 1.4 B of TMS 602-16 prior to construction. For Risk Category IV buildings this should be verified at every 5,000 sq. ft. of construction.
Verification of proportions of materials in grout and premixed or preblended mortar	-----	X	Verify that proportions for mortar meet ASTM C 270 and proportions for grout meet ASTM C 476. This applies to Risk Category IV buildings only.



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CONSTRUCTION SAFETY GENERAL NOTE

THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, TEMPORARY SHORING/BRACING, OR FOR SAFETY PRECAUTIONS AND PROGRAMS, SINCE THESE ARE SOLELY THE CONTRACTOR'S RESPONSIBILITY.

Concrete General Notes 3100:

- All detailing, fabrication and placing of reinforcing steel shall conform to the ACI Standard "Details and Detailing of Concrete Reinforcement" (ACI 315).
- Concrete at slab-on-grade shall develop a 28-day minimum compressive strength of 3,500 psi. Follow normal mixing time and speed as recommended by ASTM C94.
- All concrete at footings shall typically develop a 28-day minimum compressive strength of 3,000 psi.
- All concrete for footings and slab-on-grade shall have a 5" maximum slump.
- All reinforcing steel shall be deformed bars conforming to ASTM A615, Grade 60.
- All reinforcing bar splices shall be 44 bar diameters for #6 and smaller diameter bars. Reinforcing bar splices shall be 48 bar diameters for #7 and larger bar diameters.
- All reinforcing bar hooks shall be ACI standard 90 degree hook, unless noted otherwise.
- Provide two #4 x 4'-0" long diagonal bars centered in slab, at all re-entrant corners and any other locations designated on the plans.
- Provide corner bars in footings and turn-down slab same size and spacing as longitudinal reinforcing.
- Provide (1) #4 hoop with 8" lap in slab-on-grade around floor drains, columns and all slab penetrations 3" in diameter or greater. Also install around electrical conduit groupings 3" in diameter or greater.
- Limit the width of conduit groups to 3'-0" as they pass under wall footings. As much as possible, align the conduit perpendicular to the footing as it passes under the footing. Provide a minimum spacing of 2'-0" between conduit groups as the conduit passes under a footing. **Do not extend conduits under column footings or spread footings.**
- Plate dowel system shall be Diamond Dowel System by PNA Construction Technologies, the Speed Plate System by SIKA Corp. or approved equal. Install plate dowels at slab construction joints at 18" o.c.
- Welded wire fabric shall conform to ASTM A1064. Provide mesh in flat sheets.
- Wire fabric reinforcing shall lap 6" and be securely wired at each side and end.
- Smooth dowels shall be steel conforming to ASTM A36.
- All slots, sleeves and other embedded items shall be set before concrete is placed. See Architectural, Electrical, Mechanical, and Vendor's drawings for size and locations.
- Bar supports at footings and slab-on-grade shall be factory made wire bar supports, type "SBU" linear supports.
- Epoxy for doweling reinforcement shall be HY-200 by Hilti, AT 3G by Simpson Strong-Tie or AC200+ by Dewalt, unless noted otherwise.
- Maximum net allowable bearing pressure = 2,000 psf for continuous footings and 2,500 psf for isolated spread footings. Footings shall bear on natural, stiff to very stiff, sandy clean clay and sandy clay soils as described in the Soils Report No. A25186.00135.000 dated January 26th, 2026.
- Use of compacted, free-draining pea gravel, crushed stone, or coarse sand underneath the building slab is recommended by TSW, Inc. Consult Geotechnical Engineer regarding potential substitution of free-draining coarse materials with approved subgrade. Slabs-On-Grade have been designed for a modulus of subgrade reaction (k-value) of 100 psi/in.

NOTE:
EXPANSIVE CLAYS, LOW-STRENGTH SOILS, SOFT SOILS AND SOILS WITH ORGANIC MATERIAL ARE NOT SUITABLE IN SUPPORTING THE SLAB AND FOUNDATIONS. IF CONTRACTOR DISCOVERS UNSTABLE MATERIAL DURING EARTHWORK, A GEOTECHNICAL ENGINEER SHALL BE CONSULTED IN ORDER TO ARRIVE AT A SOLUTION THAT WILL NOT COMPROMISE THE STRUCTURAL INTEGRITY OF THE SLAB AND FOUNDATIONS.

Concrete Masonry General Notes 4100:

- All concrete masonry units shall be lightweight above finished floor and normal weight below grade. All hollow concrete masonry units shall conform to ASTM C90, Grade N, Type 1 with a minimum ultimate compressive prism strength (f'm) of 2,000 psi for the masonry assemblage. All concrete masonry shall be laid in Running (Common) Bond.
- Mortar at exterior walls, all load-bearing walls, walls below grade and non-load-bearing walls higher than 20'-0" shall be Type S mortar and have a minimum compressive strength of 1,800 psi. Mortar at interior non-load-bearing walls not higher than 20'-0" and mortar at masonry veneer shall be Type N mortar and have a minimum compressive strength of 750 psi. All mortar shall conform to ASTM C270. **Masonry cement shall not be used for mortar.**
- All grout shall be ready-mix concrete, with 3/8" diameter max. aggregate, have a minimum 28-day compressive strength of 2,000 psi and a design slump between 8" to 10" or preblended product (Core Fill Grout, Coarse CF-02, by Spec Mix) with a minimum 28-day compressive strength of 2,000 psi and a design slump between 8" to 10".
- All 8" CMU bond beam units shall be reinforced with one bar. See Masonry Wall Reinforcement Schedule on Drawing S3.0 for size of bars for vertical wall reinforcement and bond beam requirements. Provide corner bars and lap bond beam reinforcing 48 bar diameters.
- All reinforcing steel shall be deformed bars conforming to ASTM A615, Grade 60.
- All bolts, anchors, reinforcement and embedded items shall be grouted in place.
- All reinforcing bar splices shall be 48 bar diameters, U.N.O.
- At all 8" CMU walls except at interior non-load bearing walls, provide (1) vertical bar each cell for the first (2) cells adjacent to control joints in walls, at ends of walls, wall corners and on each side of wall openings, unless noted otherwise. Vertical bars shall match reinforcement for remainder of wall. See Masonry Wall Reinforcement Schedule on Drawing S3.0 for size of reinforcement.
- Provide control joints in CMU walls where shown on Drawing S3.0. Place joints for CMU walls max. 24'-0" o.c.
- Provide horizontal joint reinforcement at 16" o.c. Reinforcement shall be ladder design, min. 9 gage welded steel wire, hot dipped galvanized to 1.5 oz. width shall be 1 1/2" less than wall thickness.

NOTE:
ARCHITECT MUST APPROVE USE OF CONTROL AND CONSTRUCTION JOINTS BEFORE SLAB CONSTRUCTION BEGINS. TYPICAL SLAB CJ SHALL BE CONSTRUCTION JOINT DETAIL.

SAWCUT JOINT 1/8" x 1 1/2" DEEP TO PRODUCE VERTICAL SIDEWALL & HORIZONTAL SHELF TO SUPPORT JOINT FILLER. FILL W/ JOINT FILLER AT EXPOSED JOINTS & FILL W/ SUBFLOOR FILLER UNDER TILE & CARPET

15 MIL POLYETHYLENE - LAP 6" AND TAPE EA. END
COMPACTED FREE-DRAINING PEA GRAVEL, CRUSHED STONE, OR COARSE SAND HAVING NO MORE THAN 50% PASSING THE NO. 50 SIEVE AND NO MORE THAN 5% PASSING THE NO. 200 SIEVE. SEE CONC. GEN. NOTE #20

NOTE:
COORDINATE EXACT LOCATION AND LIMITS OF POLISHED CONCRETE WITH ARCHITECTURAL DWGS.

WWF
T.O. SLAB EL.
SEE PLAN
PROPERLY PREPARED SUBGRADE IN ACCORDANCE W/ PROJECT GEOTECHNICAL REPORT

1 TYP. SLAB-ON-GRADE CONTROL JT. DTL. (CJ)

NOT TO SCALE

NOTE:
COORDINATE LOCATION OF RECESSED SLAB WITH ARCHITECTURAL DRAWINGS.

T.O. SLAB EL.
SEE PLAN
1 6" MIN.
T.O. SLAB EL.
SEE PLAN

3 TYP. RECESSED SLAB

NOT TO SCALE

NOTE:
PROVIDE DIAMOND DOWEL SYSTEM AS MANUFACTURED BY PNA CONSTRUCTION TECHNOLOGIES OR THE SPEED SYSTEM BY SIKA CORPORATION AT CONSTRUCTION JOINTS IN ALL FLOOR AREAS. COORDINATE WITH ARCHITECT FOR CONSTRUCTION JOINT LOCATIONS.

RESAW JOINT 1/8" x 1 1/2" DEEP TO PRODUCE VERTICAL SIDEWALL & HORIZONTAL SHELF TO SUPPORT JOINT FILLER. FILL W/ JOINT FILLER AT EXPOSED JOINTS & FILL W/ SUBFLOOR FILLER UNDER TILE & CARPET

15 MIL POLYETHYLENE - LAP 6" AND TAPE EA. END
COMPACTED FREE-DRAINING PEA GRAVEL, CRUSHED STONE, OR COARSE SAND HAVING NO MORE THAN 50% PASSING THE NO. 50 SIEVE AND NO MORE THAN 5% PASSING THE NO. 200 SIEVE. SEE CONC. GEN. NOTE #20

DIAMOND DOWEL @ 18" O.C.
WWF - DISCONTINUE REINF. AT JOINT
T.O. SLAB EL.
SEE PLAN
PROPERLY PREPARED SUBGRADE IN ACCORDANCE W/ PROJECT GEOTECHNICAL REPORT

2 TYP. SLAB-ON-GRADE CONSTRUCTION JT. DTL. (CJ)

NOT TO SCALE

NOTE:
SEE FOUNDATION DETAILS FOR INFORMATION NOT SHOWN.

SEE ARCH.
(COORD. W/ EQUIP. MFR.)
#3 @ 12" O.C. EA. WAY
T.O. SLAB EL.
SEE PLAN
(2) #3 x 0'-6" L.G. DOWELS @ 48" O.C. - DRILL & EPOXY DOWELS 3" INTO TOP OF SLAB
1 1/2" CLR.
3/4" = 1'-0"

4 TYP. LOCKER PAD FOUNDATION

3/4" = 1'-0"

TYPICAL STRUCTURAL ABBREVIATIONS

A.R.	ANCHOR ROD	FDN	FOUNDATION	PED	PEDESTAL
ACI	AMERICAN CONCRETE INSTITUTE	FIN	FINISH	PL	PLATE
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	FLR	FLOOR	PLF	POUNDS PER FOOT
ARCH	ARCHITECT	FTG	FOOTING	PROJ	PROJECTION
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS	GA	GAUGE	PSF	POUNDS PER SQUARE FOOT
BLDG	BUILDING	GALV	GALVANIZED	PSI	POUNDS PER SQUARE INCH
BM	BEAM	H.S.A.	HEADED STUD ANCHOR	R	RADIUS
BOTT	BOTTOM	HK	HOOK	REINF	REINFORCEMENT
BRG	BEARING	HORIZ	HORIZONTAL	REQD	REQUIRED
BTWN	BETWEEN	J.B.E.	JOIST BEARING ELEVATION	RTU	ROOF TOP UNIT
CFS	COLD-FORMED STEEL	JST	JOIST	S.O.G.	SLAB ON GRADE
CL	CENTER LINE	JT	JOINT	SCHED	SCHEDULE
CLR	CLEAR	LDH	LONG DIMENSION HORIZONTAL	SECT	SECTION
COL	COLUMN	SEOR	STRUCTURAL ENGINEER OF RECORD	SEOR	STRUCTURAL ENGINEER OF RECORD
CONC	CONCRETE	LDV	LONG DIMENSION VERTICAL	SIM	SIMILAR
CONN	CONNECTION	LG	LONG	SJI	STEEL JOIST INSTITUTE
CONT	CONTINUOUS	LLH	LONG LENGTH HORIZONTAL	SPA	SPACING
DEFL.	DEFLECTION	SPCS	SPECIFICATIONS	STD	STANDARD
DIA, or Ø	DIAMETER	STD	STANDARD	STIFF	STIFFENER
DIM	DIMENSION	STL	STEEL	TOC	TOP OF CONCRETE
DN	DOWN	TOF	TOP OF FOOTING	TOS	TOP OF STEEL
do	DITTO	TOTB	TOP OF TIE BEAM	TOW	TOP OF WALL
DTL	DETAIL	TRANS	TRANSVERSE	TYP	TYPICAL
DWG	DRAWING	U.N.O.	UNLESS NOTED OTHERWISE	U.N.O.	UNLESS NOTED OTHERWISE
E.F.	EACH FACE	VERT	VERTICAL	W	WITH
E.W.	EACH WAY	WP	WORK POINT	WWF	WELDED WIRE FABRIC
EA	EACH				
EL	ELEVATION				
EQ	EQUAL				
EXIST	EXISTING				
EXP	EXPANSION				
F.S.	FAR SIDE				
F.V.	FIELD VERIFY				



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A NEW FACILITY FOR
BWHS - Locker Room Building
1359 Gamble Road, Centerton, AR 72719

DRAWN BY:
TMW

CHECK BY:
ASD

ISSUE DATE:
04/06/2026

PROJECT NO:
2421.2

REVISION DATES

FOUNDATION GENERAL NOTES & TYP. DETAILS
S H E E T

S1.1

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(479) 621-6128 ROGERS, ARKANSAS
TSW #: 26004 PM: ASD DE: BWA

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Structural Steel General Notes 5100:

- All detailing, fabrication and erection of structural steel shall conform to the requirements of the AISC Specification for the Design, Fabrication and Erection of Structural Steel for Buildings.
- Wide flanges and WT tees shall conform to ASTM A992 with a yield strength of 50 ksi.
- Headed stud anchors (H.S.A.'s) shall conform to ASTM A108.
- All other structural steel shall conform to the requirements of ASTM A36. Angle, plate and beam lintels at exterior wall openings shall be hot-dipped galvanized.
- All welding shall conform to the Specifications of the American Welding Society. Welding electrodes shall be E-70 low hydrogen series. Welding shall be done by a certified welder.
- High strength bolts shall typically be 3/4" diameter bolts conforming to ASTM A325. Connections shall be designed as bearing type with threads in shear plane. Holes shall be 1/16" larger than bolt size. See details for connections with 1" diameter bolts.
- All bolts shall be tightened to a snug-tight condition. A snug tight condition is defined as the tightness attained by a few impacts of an impact wrench or the full effort of a man using an ordinary spud wrench. All connected elements must be brought into snug contact.
- No openings shall be cut in structural members unless shown on the drawings.
- Steel frame is non-self-supporting and is designed for a completed condition only. Metal roof deck and masonry shear walls are required to provide lateral stability for the frame and resistance to wind and seismic forces. Contractor shall provide all temporary bracing required to maintain stability of structural system.
- All exposed edges of plates, beams, etc., shall be shop ground smooth and uniform.
- 1/2"Ø sleeve anchors shall be 1/2"Ø x 4" long HLC-H Sleeve Anchor by Hilti. HLC-H Sleeve Anchor shall be carbon steel with zinc plating & have a 3" embedment depth.
- 1/2"Ø expansion bolts shall be 1/2"Ø x 5 1/2" long Kwik Bolt 3 by Hilti. Expansion Anchor shall be carbon steel with zinc plating & have a 3 1/2" embedment depth.
- 3/4"Ø expansion bolts shall be 3/4"Ø x 5 1/2" long Kwik Bolt 3 by Hilti. Expansion Anchor shall be carbon steel with zinc plating & have a 4 3/4" embedment depth.

GENERAL CONTRACTOR SHALL INCLUDE 1 TON AT A MINIMUM COST OF \$10,000/ TON OF MISCELLANEOUS STEEL BEAMS, CHANNELS AND ANGLES IN ADDITION TO THE FRAMING SHOWN ON THE PLANS AND DETAILS. GENERAL CONTRACTOR SHALL INCLUDE ERECTION, FABRICATION, DESIGN AND DETAILING COSTS FOR THIS ADDITIONAL FRAMING WITH THE BASE BID. THE USE OF MISCELLANEOUS STEEL IS TO BE RECORDED BY THE GENERAL CONTRACTOR AND ANY UNUSED AMOUNT IS TO BE CREDITED TO THE OWNER.

Steel Deck General Notes 5300:

- Typical roof deck shall be 1 1/2" deep, 22 gauge, wide rib type and shall have nested side laps (Vulcraft 1.5B22, New Millennium 1.5B22 or approved equal). See Roof Framing Plans for limits of roof deck.
- Roof deck shall be welded to the steel framing per the Roof Deck Fastening Pattern Detail 1/S1.2.
- Roof deck fastening pattern has been designed for a net wind uplift of 23.7 psf at corner zones, 18.9 psf at side zones and 6.8 psf at interior zones for roofs.
- All deck shall be fastened per Steel Deck Institute (SDI) requirements.
- Deck specified has been determined on basis of 3 span condition; deck supplier shall use heavier gauge if required for one and two span conditions.

Light Gauge Steel General Notes 5400:

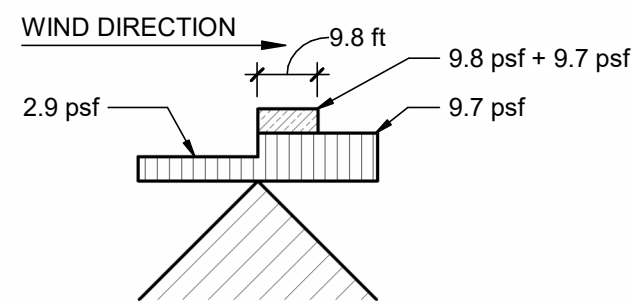
- All structural studs and headers shall be of the type, size, gauge and spacing as shown on the drawings.
- All framing members shall be formed from steel, corresponding to the requirements of ASTM C955.
- Steel for framing members shall have a minimum yield strength of 33 ksi for 43 mil. thickness or less and 50 ksi for 54 mil. or greater.
- Exterior studs shall be constructed with the following, unless noted otherwise.
 - 6" studs: 60S162-43
 - 6" track: 60T125-43
- See Architectural Drawings for summary of non-structural stud sizes; runner track gauge to match stud gauge.
- At interior stud walls extending to the underside of roof structures, provide a deflection track at the top of the wall.
- Provide bracing to structure for all stud walls and furr-downs.

Prefabricated Light Gauge Steel Truss General Notes 5410:

- The material and fabrication criteria of trusses shall meet the requirements of "Specifications for the Design of Cold Formed Steel Structural Members" by AISI.
- Truss members shall be fabricated of structural quality steel sheet with a protective G60 zinc coating per ASTM A653. Steel shall have a minimum yield strength of 40,000 psi.
- Truss members shall have a 18 gauge minimum thickness.
- Fasteners at truss connections shall be self-drilling, self-tapping screws with corrosion-resistant plated finish per the truss manufacturer's recommendations.
- During erection, care shall be exercised to keep horizontal bending of trusses to a minimum. Proper erection bridging and bracing shall be installed to hold the trusses true and plumb and in a safe condition until permanent truss bridging and decking are installed.
- Install continuous bridging and permanent bracing per the truss manufacturer's requirements.
- Repair damaged galvanized coatings on truss members with galvanizing repair paint according to ASTM A780 and the truss manufacturer's instructions.
- Truss manufacturer shall design trusses for a 20 psf dead load (10 psf top chord and 10 psf bottom chord dead load).
- Trusses shall be designed for a 27.9 psf gross wind uplift.
- Trusses shall be designed for a live load deflection of L/240 and a total deflection of L/360 for areas with ceiling attached directly to bottom chord.
- The truss manufacturer shall prepare complete fabrication and erection drawings, fully engineered and sealed by a registered structural engineer in the state of Arkansas.
- At gabled end trusses and where shown on drawings, trusses shall be sheathed with 1/2" gypsum sheathing. Attach using #10 screws @ 7" o.c. around perimeter and 12" o.c. in the field. Truss manufacturer to provide truss web members at spacing to accommodate sheathing attachment.

Design Loads

- Typical Roof Dead Load: 20 psf
- Roof Live Load: 20 psf
- Rain Intensity, (15 min, i): 6.64 in/hr
- Snow Load: 15 psf
 - Ground Snow Load:
 - Flat-roof Snow Load at main roof (P_f) = 10.4 psf
 - Slope Factor (C_s) = 0.93
 - Snow Exposure Factor (C_e) = 1.0
 - Snow Load Importance Factor (I_s) = 1.1
 - Thermal Factor (C_t) = 1.0
- Snow Drift:

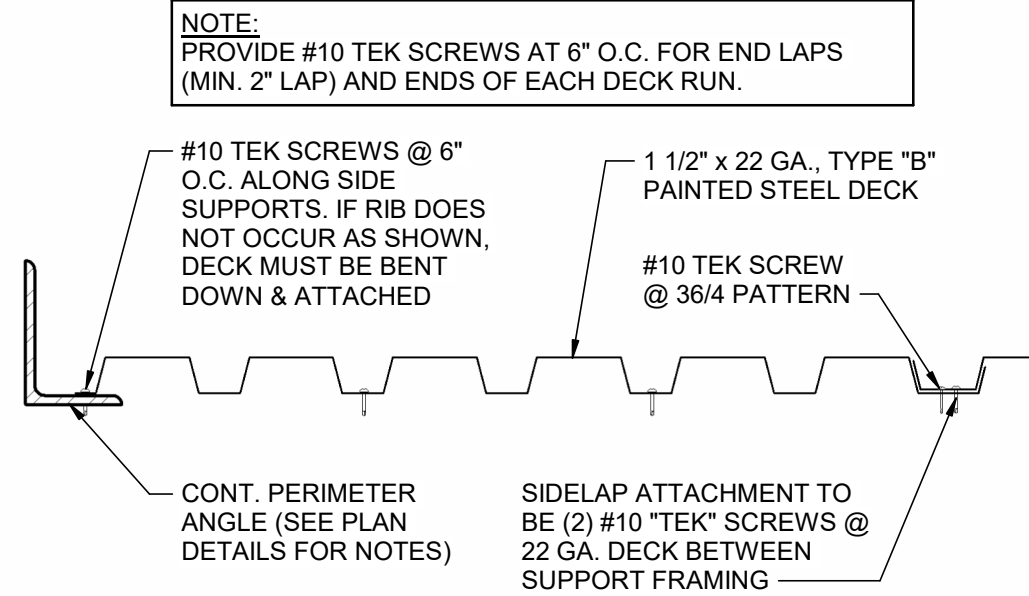


Unbalanced Snow Load

- Wind Load:
 - Ultimate Design Wind Speed (V_{ult}): 115 mph
 - Nominal Design Wind Speed (V_{nom}): 89.1 mph
 - Risk Category III
 - Wind Exposure C
 - Internal Pressure Coefficient, GC_{pi} = ± 0.18
- Components & Cladding Wind Load (Unfactored):
 - Width of Edge Zone, a = 5.80 ft
 - Wall Pressures (100 ft²)
 - End Zone Wall = 34.0 psf
 - Interior Zone = 27.6 psf
 - Wall Pressures (100 ft²)
 - End Zone Wall = 26.5 psf
 - Interior Zone = 23.9 psf
 - Roof Pressures (10 ft²)
 - Corner Zone = 89.2 psf
 - Eave & Rake Zone = 75.1 psf
 - Interior Zone = 51.5 psf
 - Roof Pressures (100 ft²)
 - Corner Zone = 46.7 psf
 - Eave & Rake Zone = 41.3 psf
 - Interior Zone = 16.1 psf

- Seismic:
 - Risk Category III
 - Seismic Importance Factor (I_h) = 1.25
 - S_s = 0.148
 - S_1 = 0.087
 - S_{os} = 0.158
 - S_{pi} = 0.140
 - Site Class D (per Geotechnical Report)
 - Seismic Design Category C
 - Basic Structural System: Bearing Wall System
 - Seismic Resisting System: Intermediate Reinforced Masonry Shear Walls
 - Response Modification Coefficient (R): 3.500
 - Deflection Amplification Factor (C_d): 2.250
 - Seismic Response Coefficient (C_s): 0.056
 - Analysis Procedure: Equivalent Lateral Force Procedure
- Building Code:
 - 2021 Arkansas Fire Prevention Code, Volume II - Adopting 2021 International Building Code
 - ASCE 7-16

THIS FACILITY HAS BEEN DESIGNED FOR THE SEISMIC CRITERIA AND BUILDING CODE NOTED ON THIS DRAWING IN ACCORDANCE WITH THE REQUIREMENTS OF ACT 1100.

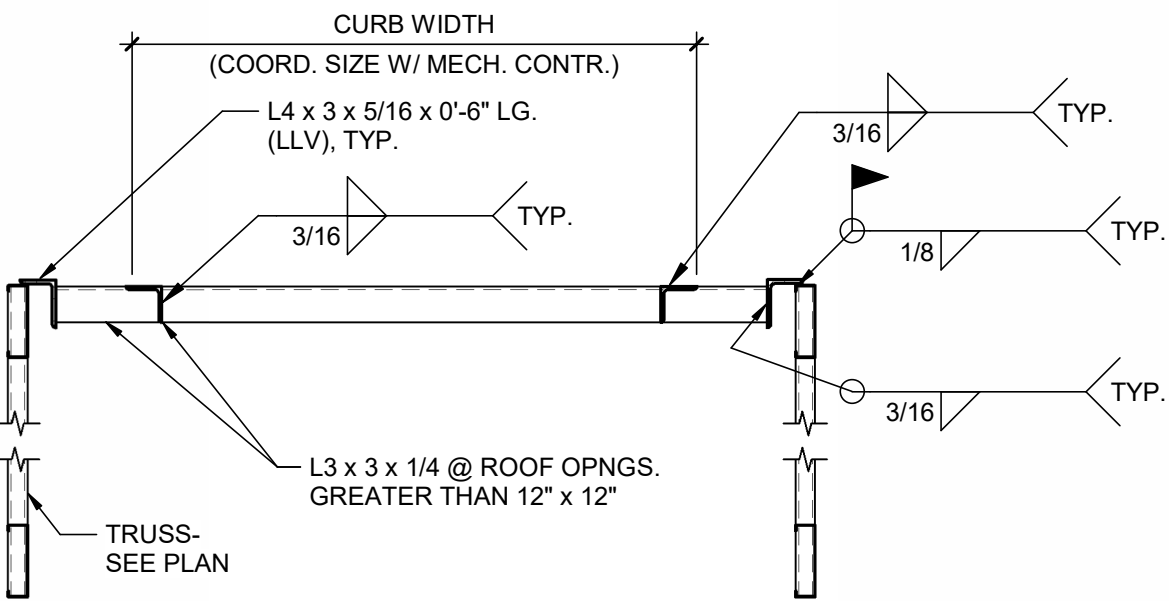


1 1/2" ROOF DECK

TYP. ROOF DECK FASTENING PATTERN DETAIL

NOT TO SCALE

NOTE:
FOR ROOF OPENINGS OVER 6" TO 12" WIDE (I.E.: OVER-SIZED OPENING FOR ROOF DRAINS), PROVIDE L2 x 2 x 1/4 FRAME AROUND OPENING AND EXTEND TO JOISTS- WELD ALL AROUND.

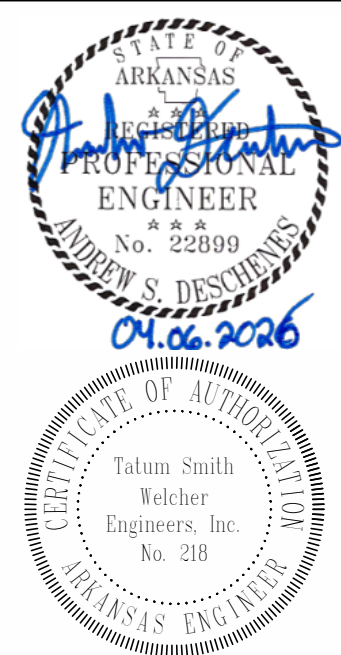


TYP. ROOF FRAMED OPNG. DETAIL

NOT TO SCALE

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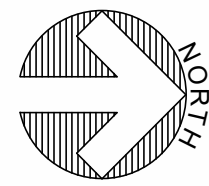
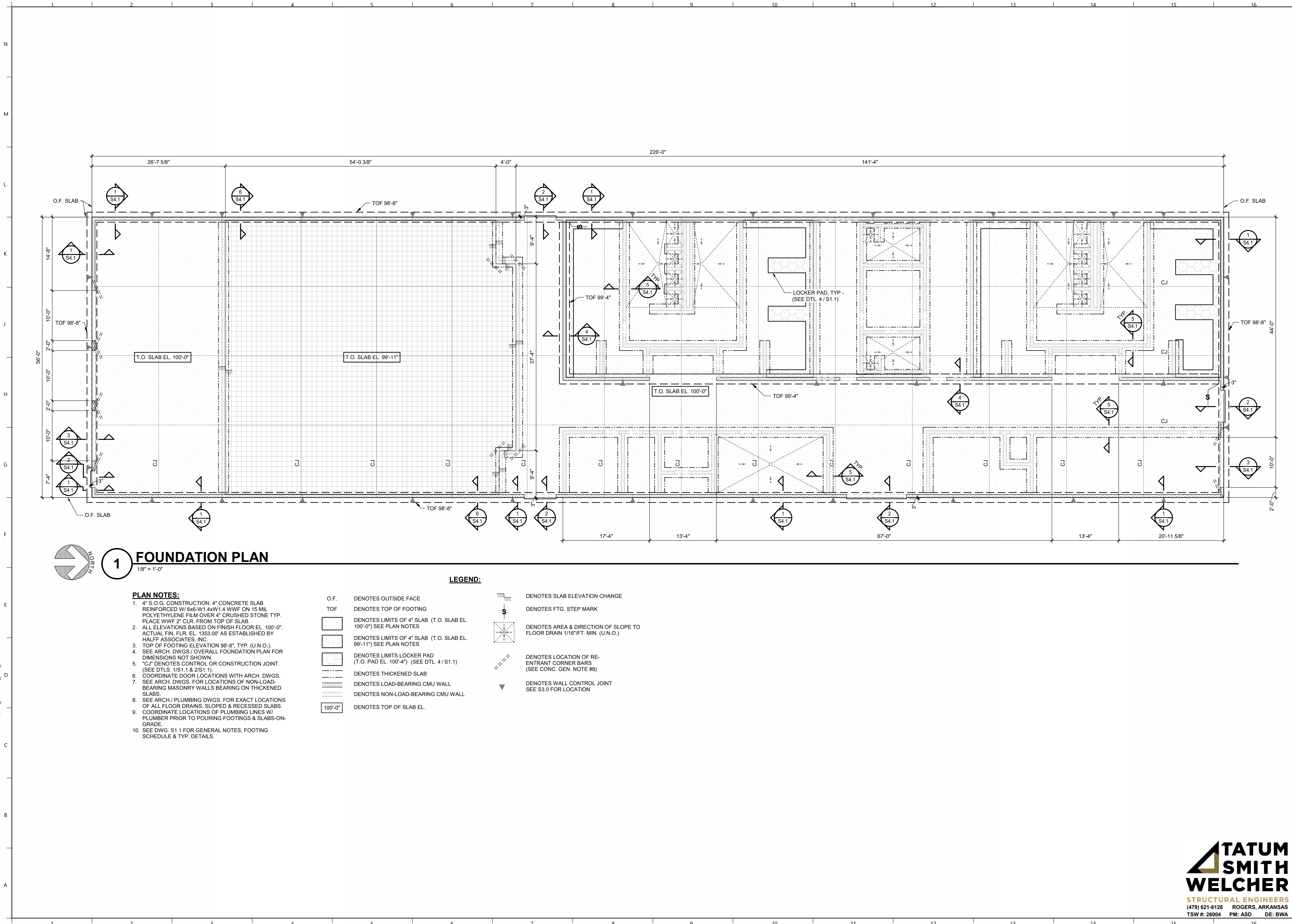
REVISION DATES

FRAMING GENERAL NOTES
& TYP. DETAILS
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S1.2

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1 FOUNDATION PLAN

1/8" = 1'-0"

PLAN NOTES:

- 4" S.O.G. CONSTRUCTION: 4" CONCRETE SLAB REINFORCED W/ 6x6-W1.4xW1.4 WWF ON 15 MIL POLYETHYLENE FILM OVER 4" CRUSHED STONE TYP. PLACE WWF 2" CLR. FROM TOP OF SLAB.
- ALL ELEVATIONS BASED ON FINISH FLOOR EL. 100'-0". ACTUAL FIN. FLR. EL. 1353.00' AS ESTABLISHED BY HALFF ASSOCIATES, INC.
- TOP OF FOOTING ELEVATION 98'-8", TYP. (U.N.O.).
- SEE ARCH. DWGS / OVERALL FOUNDATION PLAN FOR DIMENSIONS NOT SHOWN.
- "C.J." DENOTES CONTROL OR CONSTRUCTION JOINT. (SEE DTLs. 1/S1.1 & 2/S1.1).
- COORDINATE DOOR LOCATIONS WITH ARCH. DWGS.
- SEE ARCH. DWGS. FOR LOCATIONS OF NON-LOAD-BEARING MASONRY WALLS BEARING ON THICKENED SLABS.
- SEE ARCH. / PLUMBING DWGS. FOR EXACT LOCATIONS OF ALL FLOOR DRAINS, SLOPED & RECESSED SLABS.
- COORDINATE LOCATIONS OF PLUMBING LINES W/ PLUMBER PRIOR TO POURING FOOTINGS & SLABS-ON-GRADE.
- SEE DWG. S1.1 FOR GENERAL NOTES, FOOTING SCHEDULE & TYP. DETAILS.

O.F.	DENOTES OUTSIDE FACE
TOF	DENOTES TOP OF FOOTING
	DENOTES LIMITS OF 4" SLAB (T.O. SLAB EL. 100'-0") SEE PLAN NOTES
	DENOTES LIMITS OF 4" SLAB (T.O. SLAB EL. 99'-11") SEE PLAN NOTES
	DENOTES LIMITS LOCKER PAD (T.O. PAD EL. 100'-4") (SEE DTL. 4 / S1.1)
	DENOTES THICKENED SLAB
	DENOTES LOAD-BEARING CMU WALL
	DENOTES NON-LOAD-BEARING CMU WALL
100'-0"	DENOTES TOP OF SLAB EL.

LEGEND:

	DENOTES SLAB ELEVATION CHANGE
	DENOTES FTG. STEP MARK
	DENOTES AREA & DIRECTION OF SLOPE TO FLOOR DRAIN 1/16"/FT. MIN. (U.N.O.)
	DENOTES LOCATION OF RE-ENTRANT CORNER BARS (SEE CONC. GEN. NOTE #8)
	DENOTES WALL CONTROL JOINT SEE S3.0 FOR LOCATION



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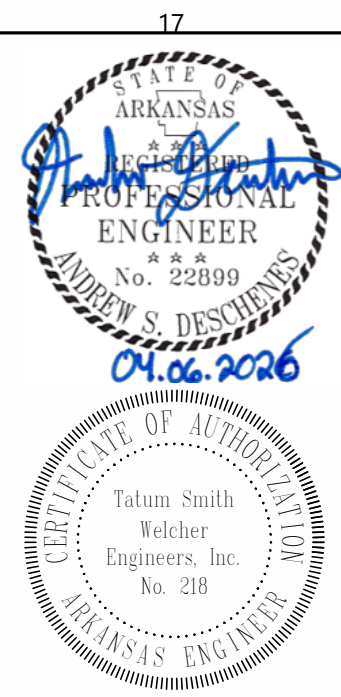
FOUNDATION PLAN

SHEET

S2.1

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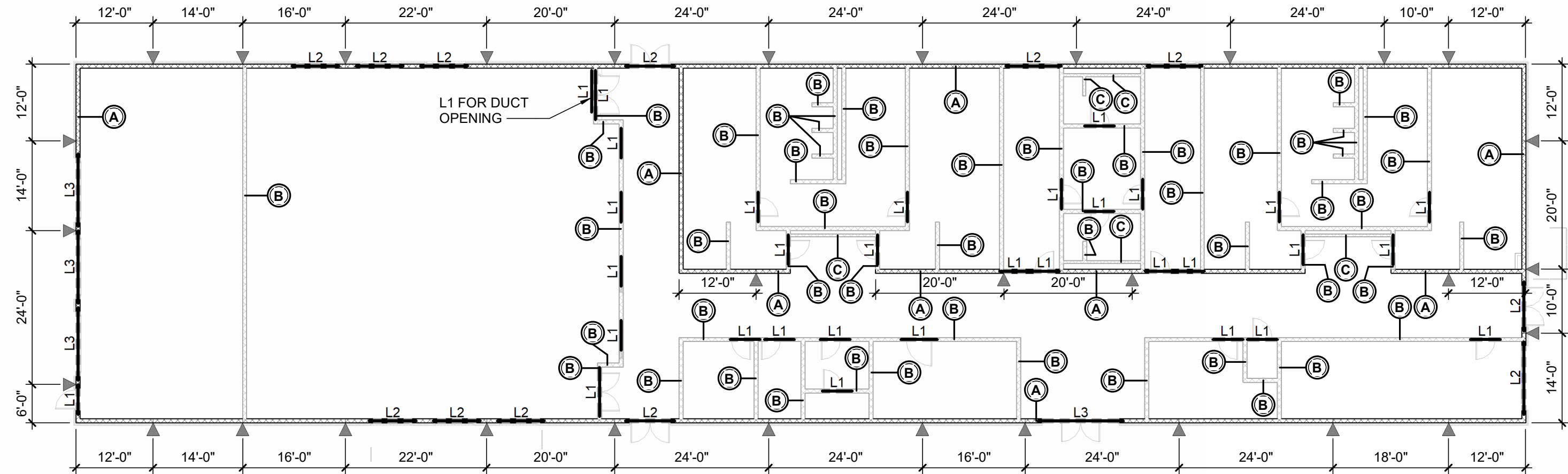
LINTEL SCHEDULE					
MARK	WALL LOCATIONS	TYPE & SIZE (THICKNESS x HEIGHT)	REINFORCEMENT OR ATTACHMENT	BRICK ANGLE OR PLATES	REMARKS
L1	8" CMU	CMU 8" x 8" BOND BM.	(1) #5 BOTT.	----	SEE NOTES #1, #2, #3, #4, #5, & #6
L2	8" CMU	CMU 8" x 8" OPEN BOTT. BOND BM. ON 8" x 8" CLOSED BOTT. BOND BM. (16" TOTAL DEPTH)	(1) #4 TOP & BOTTOM	----	SEE NOTES #1, #2, #3, #4, #5, & #6
L3	8" CMU	(2) CMU 8" x 8" OPEN BOTT. BOND BM. ON 8" x 8" CLOSED BOTT. BOND BM. (24" TOTAL DEPTH)	(1) #5 TOP & BOTTOM	----	SEE NOTES #1, #2, #3, #4, #5, & #6

- LINTEL SCHEDULE NOTES:
- SEE ARCH. DWGS. FOR EXACT LOCATION OF ALL LINTELS.
 - COORDINATE ALL BOTTOM PLATE/BEAM/CMU ANGLE ELEVATIONS WITH ARCH. DWGS.
 - ALL CMU LINTELS SHALL HAVE 8" MIN. BEARING EACH SIDE OF OPENING.
 - FILL ALL CMU LINTELS WITH 2,000 PSI GROUT.
 - ALL 8" CMU LINTELS SHALL HAVE A MIN. (2) CELLS OF (1) #5 JAMB STEEL EACH SIDE OF OPENING.
 - WHERE MECHANICAL DUCTS PASS THROUGH MASONRY WALLS, PROVIDE L1 LINTEL AT 8" CMU WALLS FOR OPENINGS NOT TO EXCEED 4'-0" WIDE. FOR OPENING WIDTHS FROM 4'-0" TO 6'-0", PROVIDE L2 LINTEL AT 8" CMU WALLS. FOR OPENING WIDTHS LARGER THAN 6'-0", COORDINATE WITH ARCH./ENGINEER.

MASONRY WALL REINFORCEMENT SCHEDULE						
MARK	WALL LOCATION	BOND BEAM REINF.	BOND BEAM LOCATIONS	VERT. REINF.	FOUNDATION DOWELS	REMARKS
A	8" CMU	(1) #5 BOTT.	TOP OF WALL, ROOF EL., & WHERE NOTED ON DWGS.	#5 @ 48" O.C.	#5 DOWEL @ 48" O.C. CENTER IN WALL	SEE NOTE #2
B	8" CMU	(1) #5 BOTT.	TOP OF WALL	-----	#4 x 2'-6" DOWEL @ 48" O.C. CENTER IN WALL	SEE NOTES #1, #3, & DTL. 2/S3.0
C	4" CMU	(1) #4 BOTT.	TOP OF WALL	-----	#3 x 1'-6" DOWEL @ 48" O.C. CENTER IN WALL	SEE NOTES #1, #3 & DTL. 2/S3.0

- MASONRY WALL REINFORCEMENT SCHEDULE NOTES:
- WHERE TOP OF WALL IS UNSUPPORTED BY THE ROOF, BRACE TOP OF WALL WITH DIAGONAL L3 x 3 x 1/4 WELDED TO THE ROOF FRAMING ABOVE AT 8'-0" O.C. MAXIMUM (SEE DTL. 2/S3.0). WHERE WALL EXTENDS TO UNDERSIDE OF ROOF DECK ABOVE, HOLD WALL DOWN 1/2" & SANDWICH TOP OF WALL WITH (2) L2 x 2 x 1/8 x 0'-6" LONG EACH ATTACHED TO DECK WITH (2) #8 SELF-TAPPING SCREWS (SPACE ANGLES AT 4'-0" O.C.) (SEE DTL. 3/S3.0). THIS BRACING IS PERMANENT BRACING REQUIRED FOR SEISMIC LOADS.
 - UNO AT SLAB-ON-GRADE, DRILL & EPOXY DOWELS 8" INTO SLAB.
 - UNO AT SLAB-ON-GRADE, DRILL & EPOXY DOWELS 6" INTO SLAB.

BEARING PLATE SCHEDULE					
LINTEL/BEAM SIZE	LINTEL BRG. PL SIZE (L x B x D)	BEAM BRG. PL SIZE (L x B x D)	HEADED STUD ANCHORS	SEE DTL.	REMARKS
ALL BEAM SIZES	3/8" x 7" x 11"	3/8" x 7" x 11"	(2) 1/2"Ø x 6" H.S.A.	5/S3.0	8" CMU WALL



1 LINTEL & MASONRY PLAN

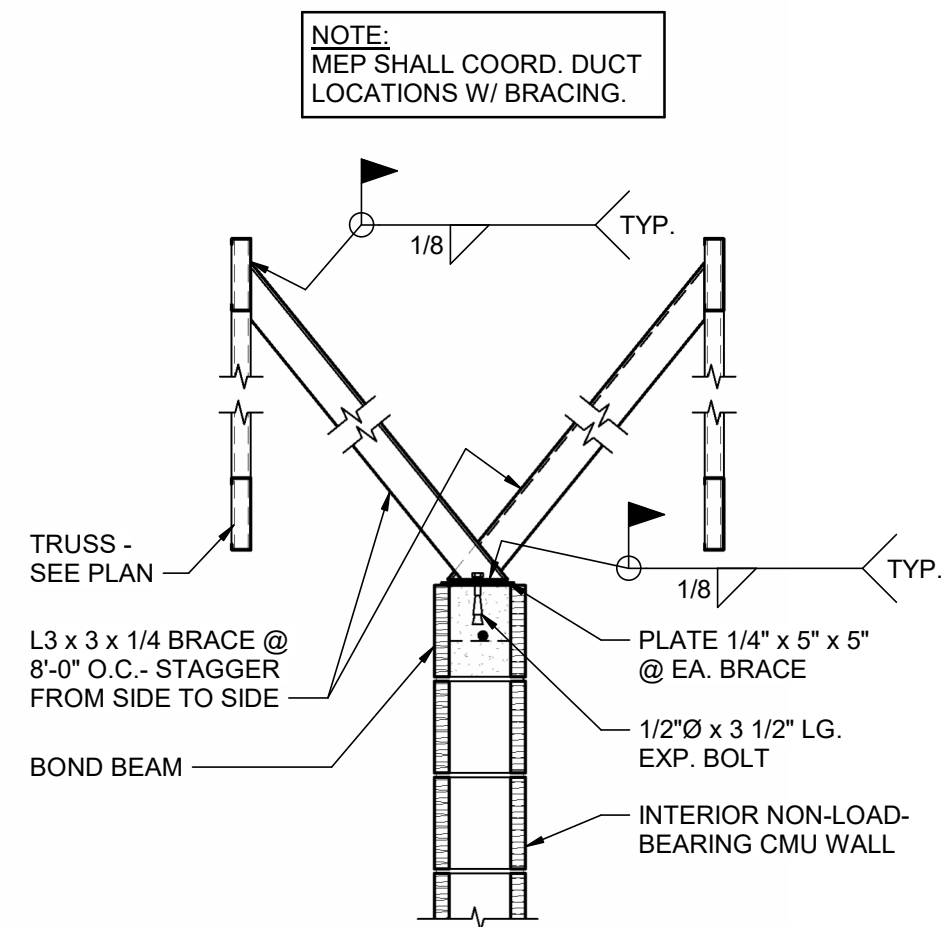
1/16" = 1'-0"

PLAN NOTES:

- SEE DWGS. S1.1 & S1.2 FOR GENERAL NOTES & TYP. DETAILS.

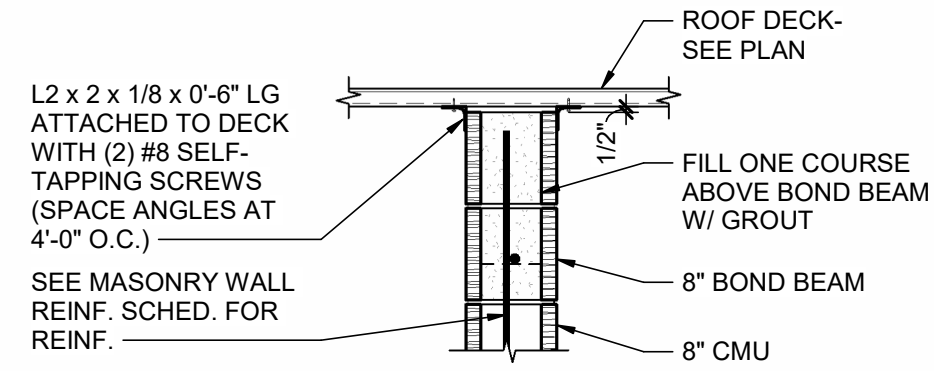
LEGEND:

- L1 DENOTES LINTEL MARK, SEE LINTEL SCHED.
- DENOTES LOAD-BEARING CMU WALL
- DENOTES NON-LOAD-BEARING CMU WALL
- (A) DENOTES MASONRY WALL REINF. MARK, SEE MASONRY WALL REINF. SCHED.
- ▼ DENOTES WALL CONTROL JOINT



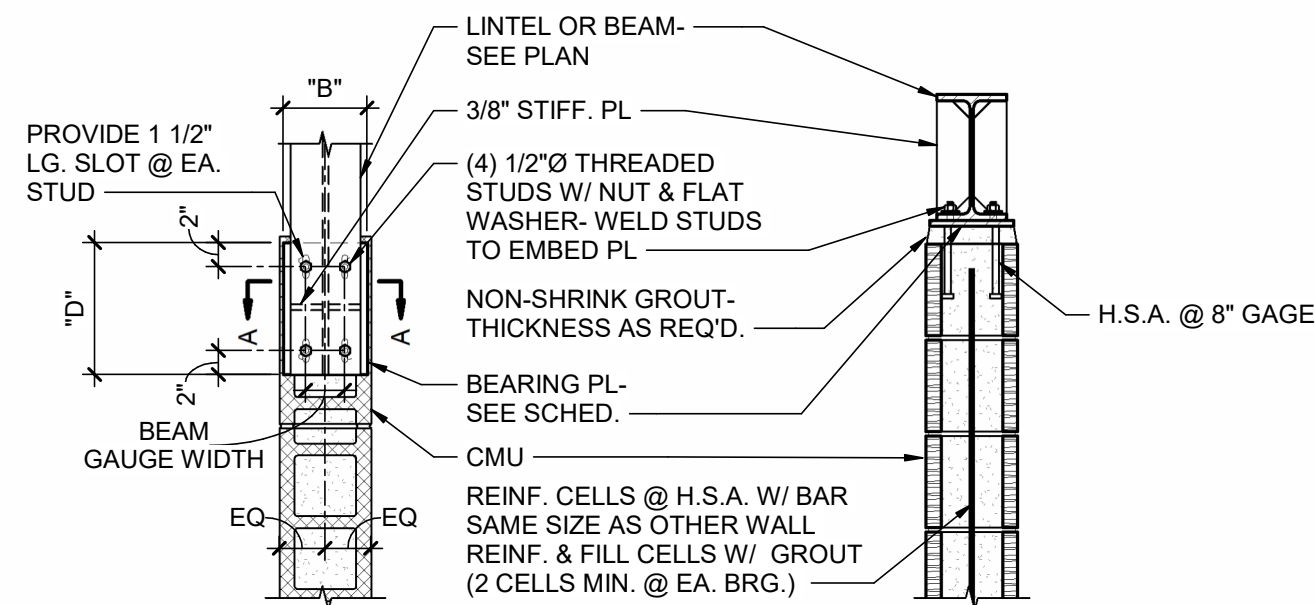
2 TYP. CMU WALL BRACING

NOT TO SCALE



3 TYP. INTR. 8" CMU WALL @ ROOF DECK

NOT TO SCALE

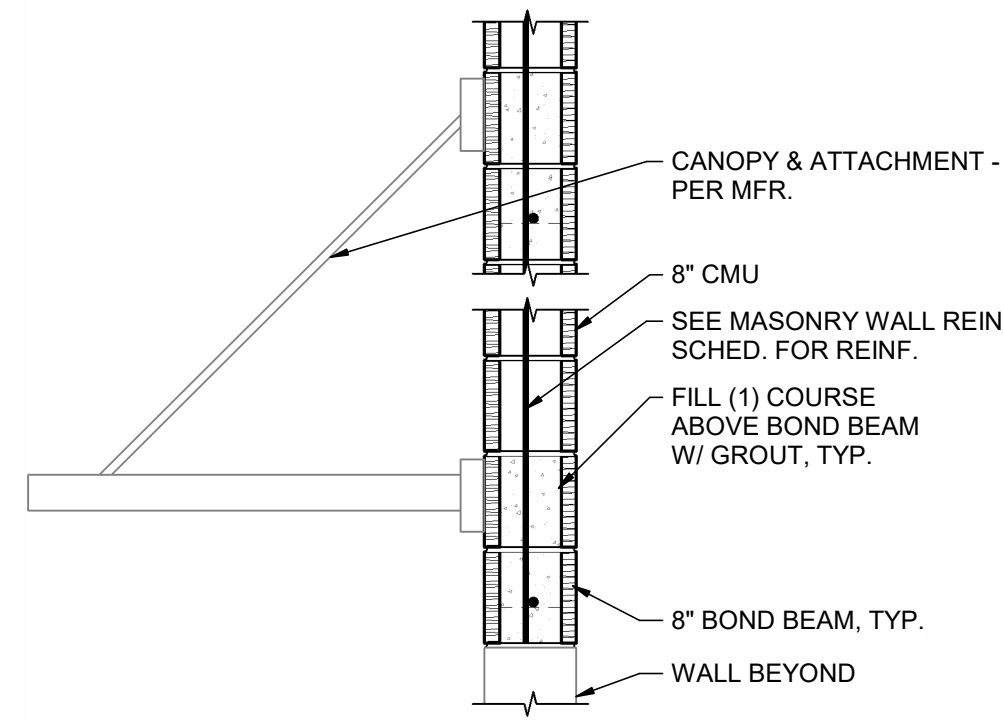


PLAN

SECTION A-A

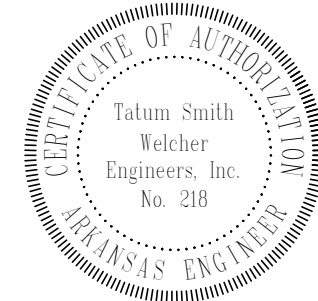
4 TYP. WF LINTEL & BEAM BEARING PLATE DETAILS

NOT TO SCALE



5 TYP. CANOPY CONNECTION PLATE DETAILS

NOT TO SCALE



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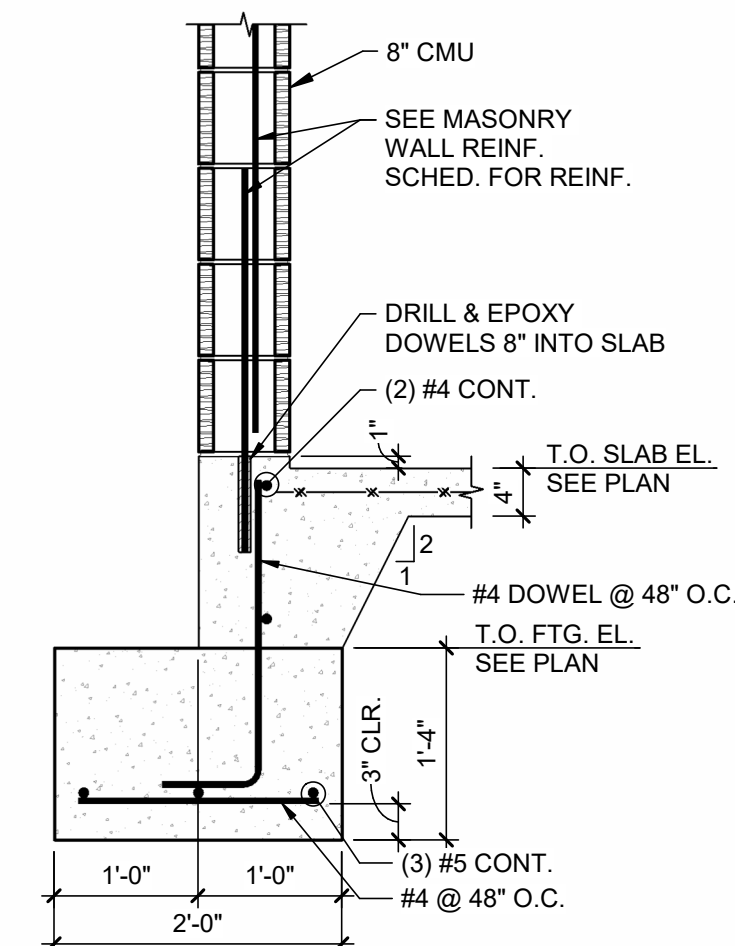
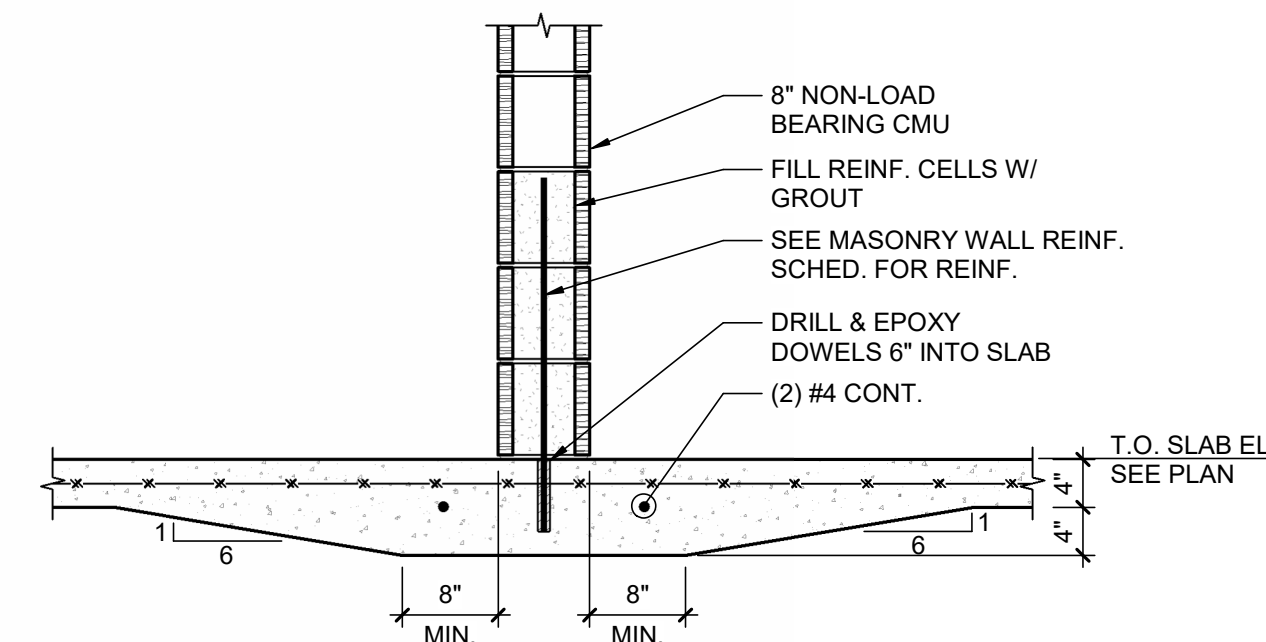
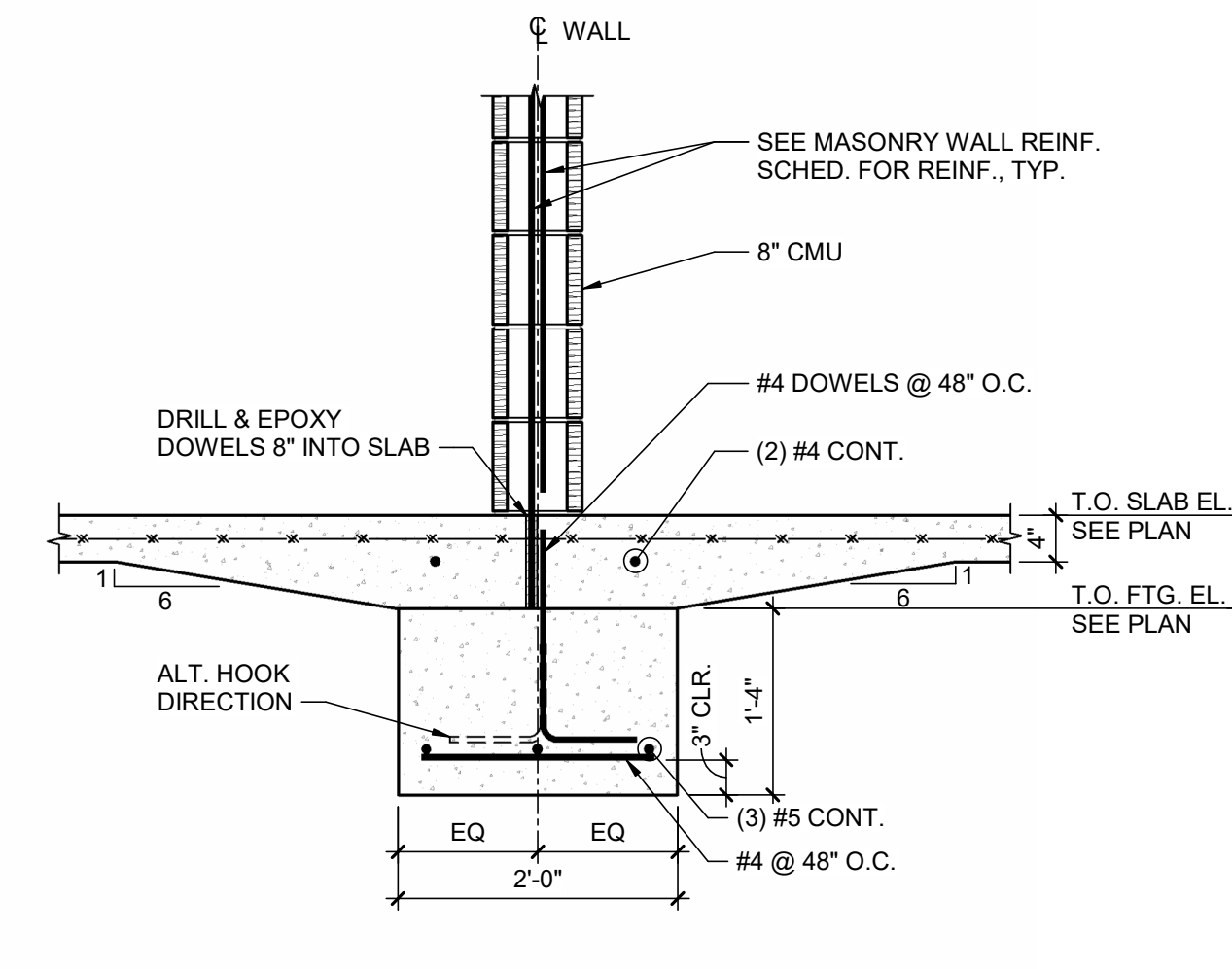
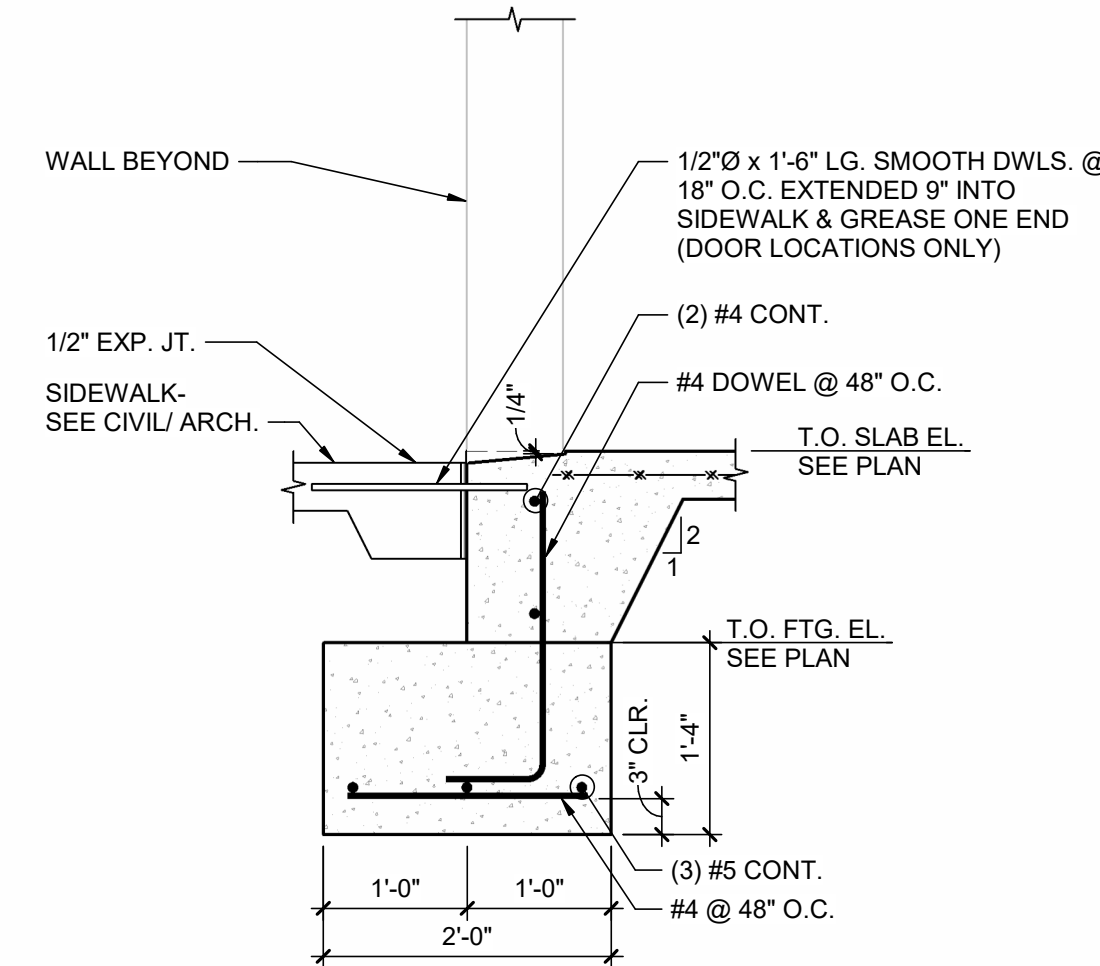
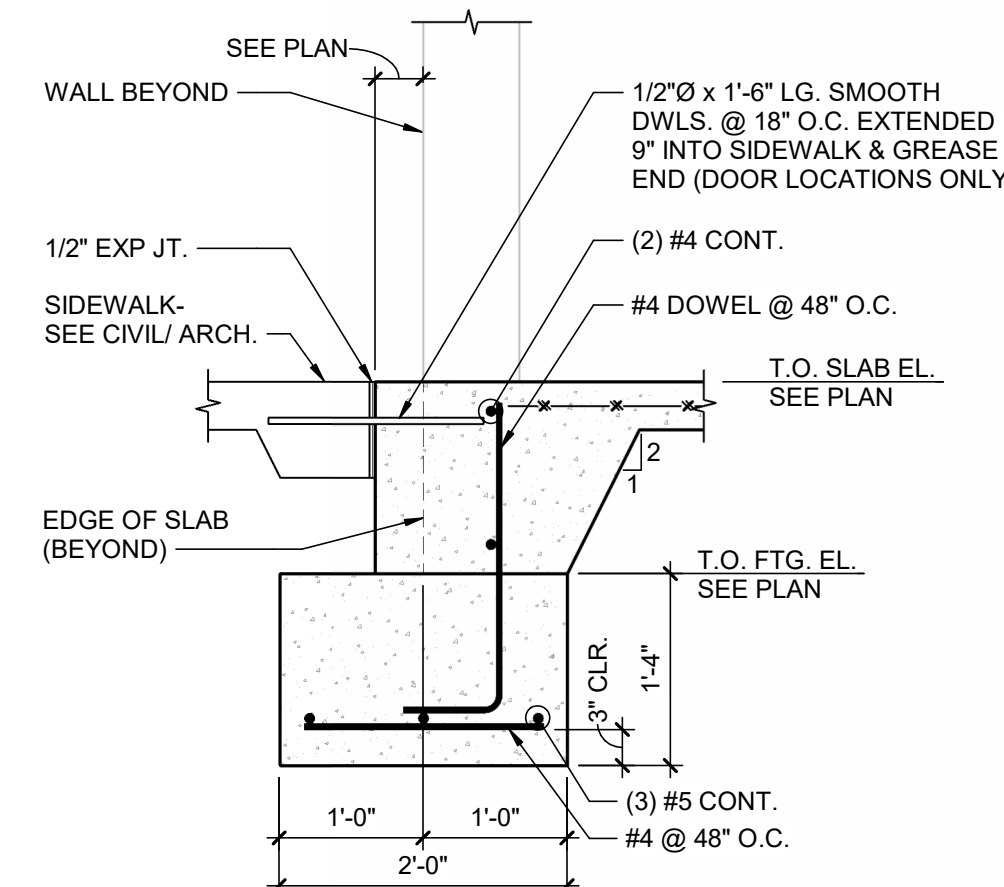
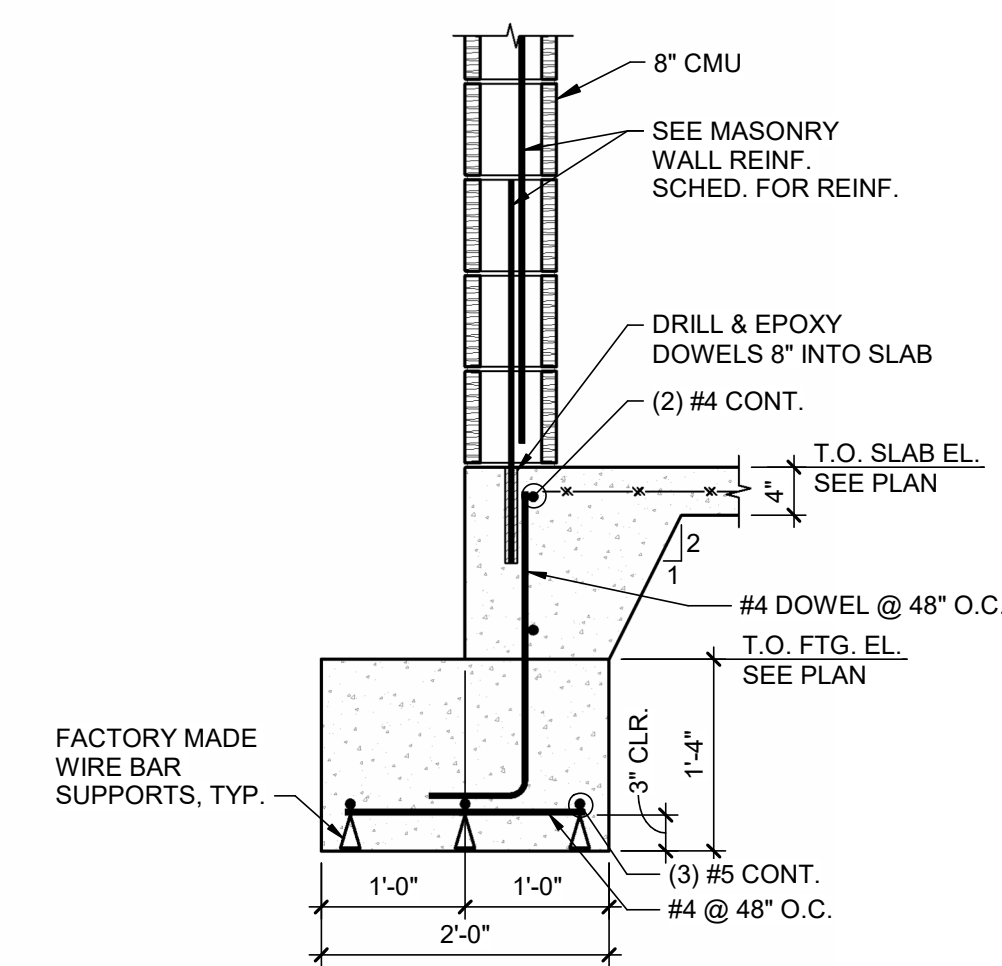
LINTEL & MASONRY PLAN
& TYP. LINTEL & MASONRY
DTLS
S H E E T

S3.0

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A NEW FACILITY FOR
BWHS - Locker Room Building
1359 Gamble Road, Centerton, AR 72719

DRAWN BY:
TMW

CHECK BY:
ASD

ISSUE DATE
4/06/2026

PROJECT NO.
2421.2

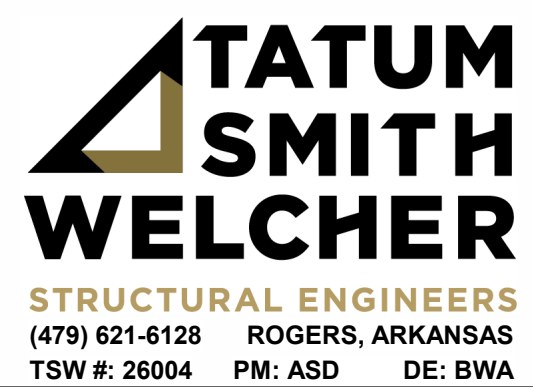
REVISION DATES

FOUNDATION DETAILS

S H E E

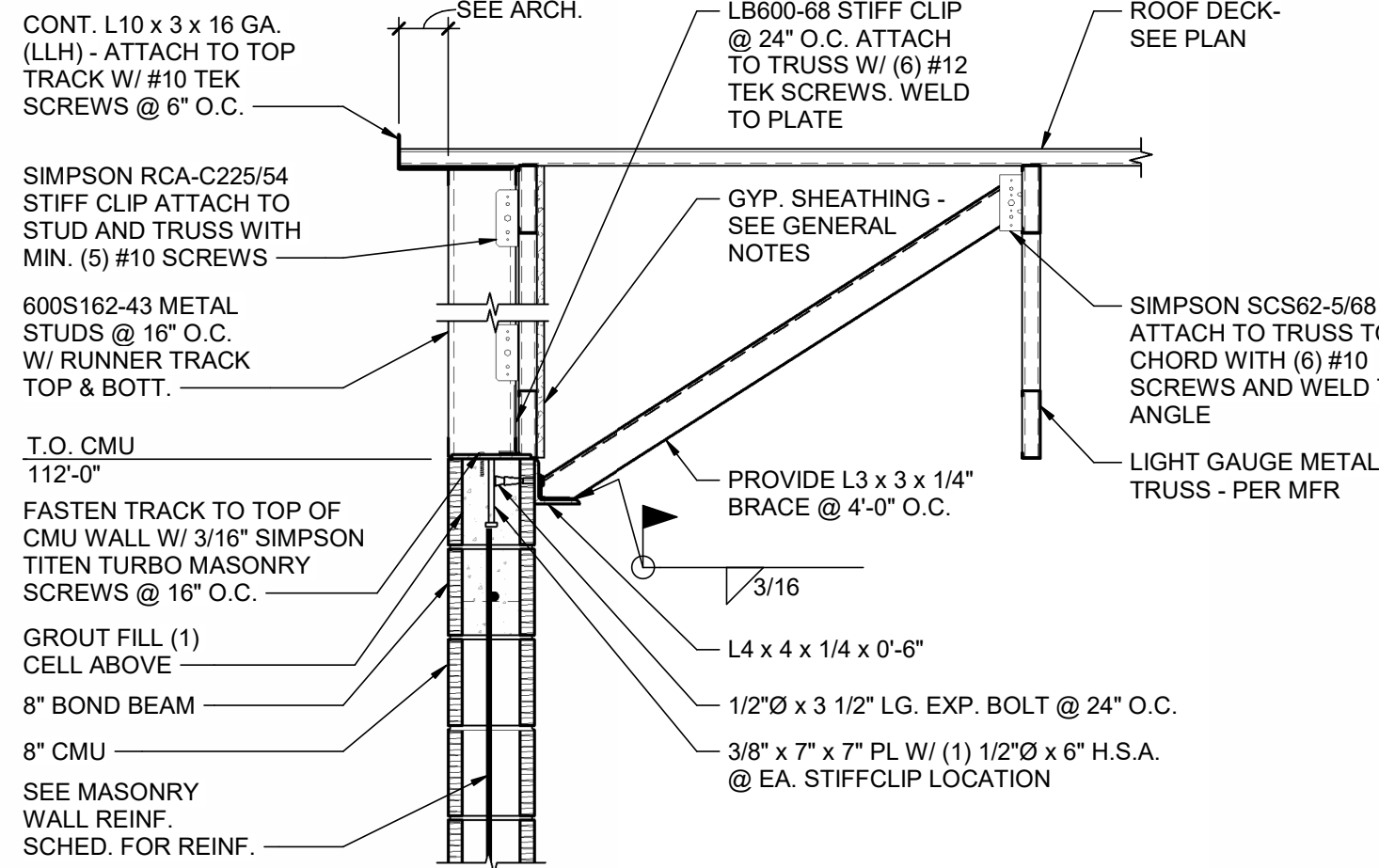
S4.1

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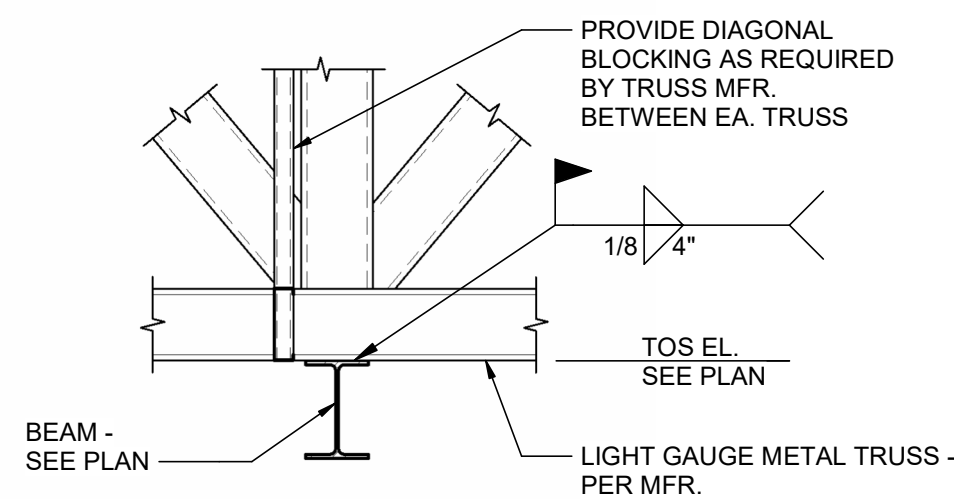
4/6/2026 8:53:44 AM
C:\Users\asad\Documents\26004 - BWHS Arts & Athletics Addition - Locker - Bentonville, AR - STRUCT_LR25_ASDMTXR7.rvt

TRUSS MFR. NOTE:
TRUSS MUST BE DESIGNED TO
TRANSFER AXIAL LOAD TO THE
CMU SHEAR WALLS INCLUDING
UNFACTORED 10.4 KIPS OF WIND
AND 7.2 KIPS OF SEISMIC.



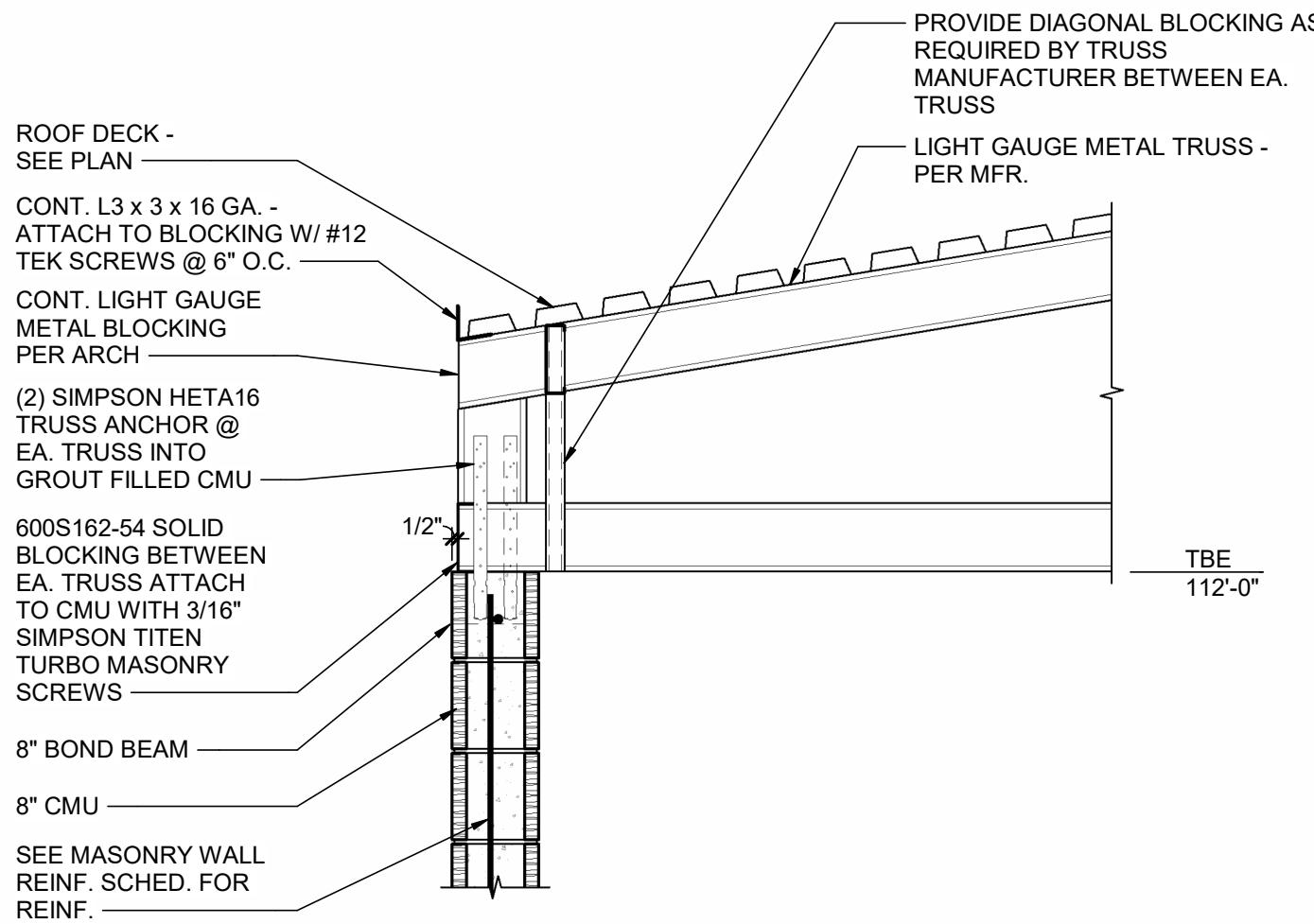
1 SECTION

3/4" = 1'-0"



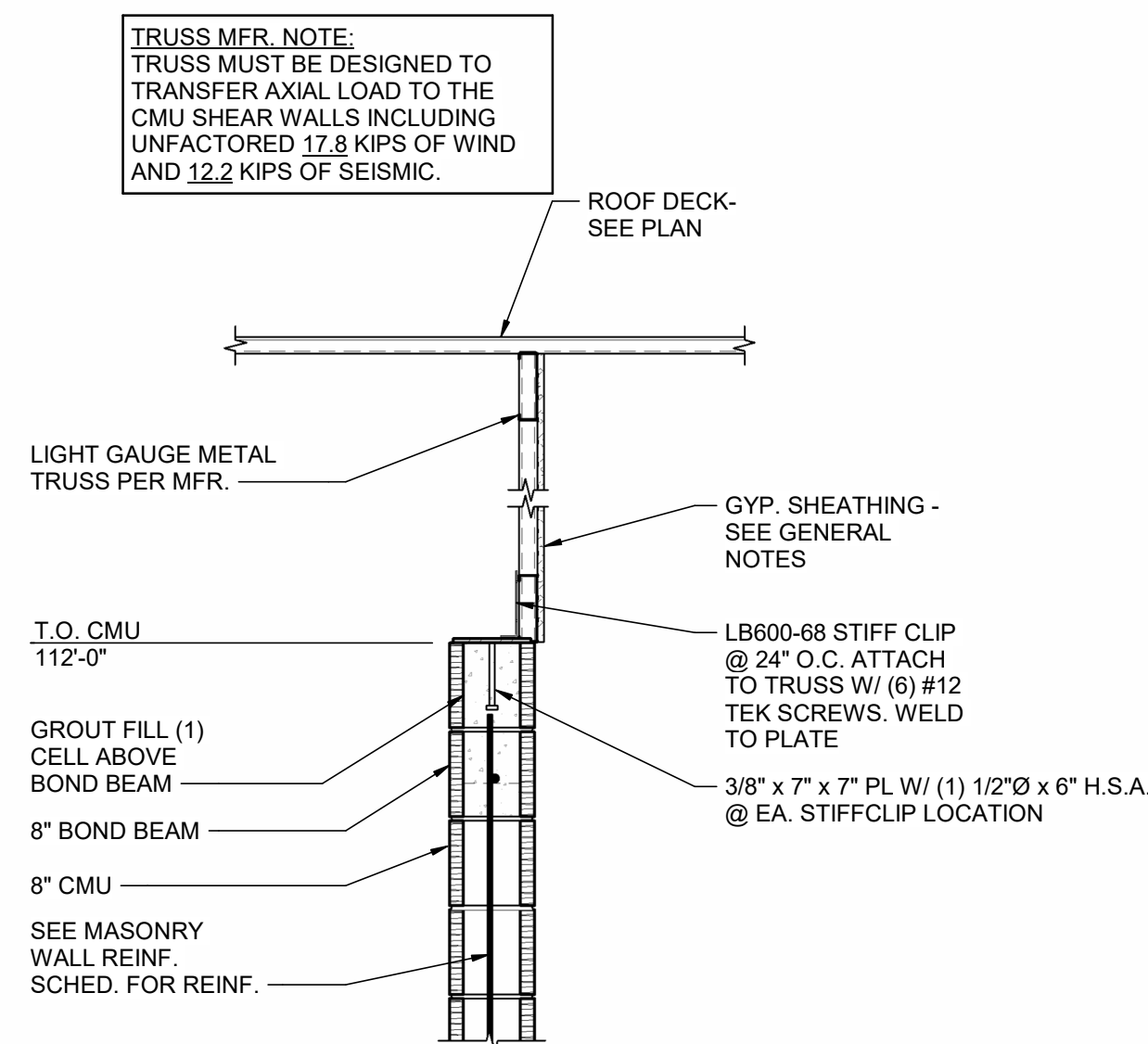
4 SECTION

3/4" = 1'-0"



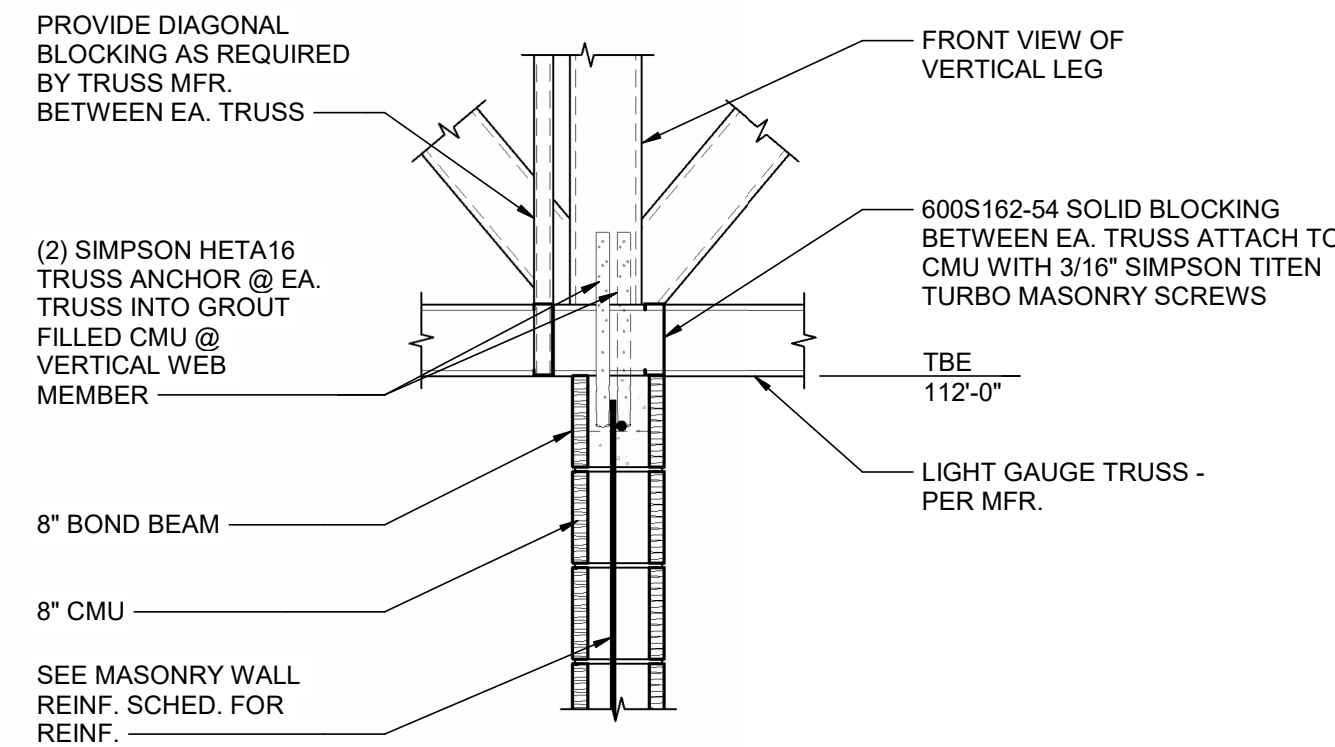
2 SECTION

3/4" = 1'-0"



5 SECTION

3/4" = 1'-0"

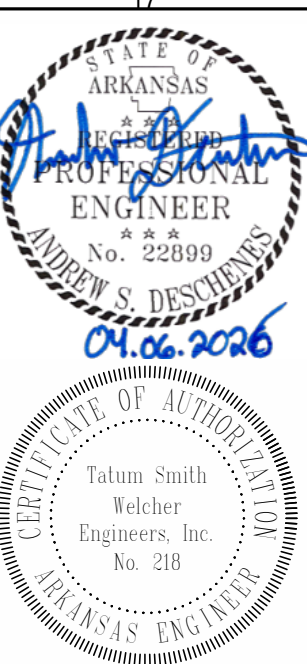


3 SECTION

3/4" = 1'-0"

TATUM SMITH WELCHER

STRUCTURAL ENGINEERS
(479) 621-6128 ROGERS, ARKANSAS
TSW #: 26004 PM: ASD DE: BWA



Hight Jackson ASSOCIATES
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DRAWN BY:
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REVISION DATES

FRAMING DETAILS

SHEET

S5.1

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